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Buckling of Beam-Columns – II

Prof. Tzuyang Yu

Structural Engineering Research Group (SERG) Department of Civil and Environmental Engineering University of Massachusetts Lowell Lowell, Massachusetts

SERG

Outline

- Method of superposition
 - To determine the maximum moment:
- Simply-supported beam-column with a uniformly distributed load and a concentrated load
 - To determine fixed-end moments:
 - Fixed-end beam-columns with a uniformly distributed load
 - Fixed-end beam-columns with a uniformly distributed load and a concentrated load
- Differential equation approach
 - Second-order vs. fourth-order
- Slope-deflection and modified slope-deflection equations
- Inelastic beam-columns
- Summary

- Method of superposition for determining the maximum moment
 - Simply-supported beam-column with a uniformly distributed load and a concentrated load

- Method of superposition for determining fixed-end moments
 - Fixed-end beam-columns with a uniformly distributed load

• Method of superposition for determining fixed-end moments

Fixed-end beam-columns with a uniformly distributed load and a concentrated load

- Differential equation approach
 - Second-order vs. fourth order

Fixed-end beam-column with a concentrated load at mid-span:
– Governing equation:

– Maximum internal bending moment: M_{max}

Fixed-end beam-column with uniformly distributed loads:
– Governing equation:

– Maximum internal bending moment: M_{max}

- Fixed-end beam-column with a concentrated bending moment at midspan:
 - Governing equation:

– Maximum internal bending moment: M_{max}

- Slope-deflection equations
 - Assumptions:
 - Prismatic beam
 - No relative joint displacement (no sidesway)
 - Continuous member (no internal hinges)
 - No in-span transverse loadings
 - Compressive axial force

- Slope-deflection equations
 - Deflection function for a beam-column

– Stability functions: **S**_{ii}

- Modified slope-deflection equations
 - Members with **sidesway**

- Modified slope-deflection equations
 - Members with rotational springs at the ends

- Modified slope-deflection equations
 - Members with **transverse loadings**

- Modified slope-deflection equations
 - Members with **tensile axial force**

Modified slope-deflection equations

– Members bent in **double curvature with** $\theta_A = \theta_B$

Inelastic beam-columns • $-M-\Phi_A$ relationship Moment M $M_{\rm pl}$ M_p $M_y = \frac{2}{3} M_{pl}$ Plastic Fully plastic М Μ. Elastic-plastic Elastic-plastic Elastic Elastic Φ, 0 0 ф Curvature ø

- Inelastic beam-columns
 - $-M \Phi_A P$ relationship



- Inelastic beam-columns
 - Case 1: Elastic

- Inelastic beam-columns
 - Case 2: Primary plastic

- Inelastic beam-columns
 - Case 3: Secondary plastic

- The general governing equation of an elastic beam-column is a fourth-order linear D.E.
- In the region of constant shear force, the use of a third-order linear D.E. is more convenient.
- In the case of an constant external moment, the use of a second-order linear D.E. is more convenient.